

Work no. 07-153	Bjørndalen Panorama Asker, Norway	Prepared by: TJP	Date: 3-maj-09	Page: 7

Cantilevered UNP320 beam - with angled cut.

Point load: $F_5 = 95 \text{ kN}$ $l = 1100 \text{ mm}$

Data for the sections:

- Section 1:	$x = 0 \text{ mm}$				
	$I_y = 8,32E+06 \text{ mm}^4$	$M_1 = 0,0 \text{ kNm}$			
	$W_y = 1,36E+05 \text{ mm}^3$	$V_1 = 95,0 \text{ kN}$			
	$A_1 = 5,84E+03 \text{ mm}^2$				
- Section 2:	$x = 200 \text{ mm}$				
	$I_y = 2,02E+07 \text{ mm}^4$	$M_2 = 19,0 \text{ kNm}$			
	$W_y = 2,32E+05 \text{ mm}^3$	$V_2 = 95,0 \text{ kN}$			
	$A_2 = 6,45E+03 \text{ mm}^2$				
- Section 3:	$x = 400 \text{ mm}$				
	$I_y = 3,81E+07 \text{ mm}^4$	$M_3 = 38,0 \text{ kNm}$			
	$W_y = 3,39E+05 \text{ mm}^3$	$V_3 = 95,0 \text{ kN}$			
	$A_3 = 7,07E+03 \text{ mm}^2$				
- Section 4:	$x = 600 \text{ mm}$				
	$I_y = 6,28E+07 \text{ mm}^4$	$M_4 = 57,0 \text{ kNm}$			
	$W_y = 4,57E+05 \text{ mm}^3$	$V_4 = 95,0 \text{ kN}$			
	$A_4 = 7,69E+03 \text{ mm}^2$				
- Section 5:	$x = 800 \text{ mm}$				
	$I_y = 9,47E+07 \text{ mm}^4$	$M_5 = 76,0 \text{ kNm}$			
	$W_y = 5,86E+05 \text{ mm}^3$	$V_5 = 95,0 \text{ kN}$			
	$A_5 = 8,30E+03 \text{ mm}^2$				
- Section 6:	$x = 1000 \text{ mm}$				
	$I_y = 1,35E+08 \text{ mm}^4$	$M_6 = 95,0 \text{ kNm}$			
	$W_y = 7,24E+05 \text{ mm}^3$	$V_6 = 95,0 \text{ kN}$			
	$A_6 = 8,92E+03 \text{ mm}^2$				

Calculation of strength and stiffness:

$$\begin{aligned} \text{Stress: } \sigma_x &= M_x / W_y \\ \tau_x &= F_5 / A_x \end{aligned}$$

$$\text{Deflection: } u_{\max} = 1,2 \cdot \frac{1}{3} \cdot \frac{F_5 \cdot l^3}{E \cdot I} \quad (*)$$

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Cantilevered UNP320 beam - with angled cut, cont.

Demands: $\sigma_- < f_{yd} = 201 \text{ MPa}$

$$\tau_- = \frac{\sqrt{3}}{3} \cdot f_{yd} = 116 \text{ MPa}$$

$$u_{\max} < l / 200 = 5,50 \text{ mm}$$

- Section 1: $\sigma_- = 0 \text{ MPa} \Rightarrow \text{OK!}$
 $\tau_- = 16,3 \text{ MPa} \Rightarrow \text{OK!}$

- Section 2: $\sigma_- = 82,0 \text{ MPa} \Rightarrow \text{OK!}$
 $\tau_- = 14,7 \text{ MPa} \Rightarrow \text{OK!}$

- Section 3: $\sigma_- = 112,2 \text{ MPa} \Rightarrow \text{OK!}$
 $\tau_- = 13,4 \text{ MPa} \Rightarrow \text{OK!}$

- Section 4: $\sigma_{-4} = 124,8 \text{ MPa} \Rightarrow \text{OK!}$
 $\tau_- = 12,4 \text{ MPa} \Rightarrow \text{OK!}$

- Section 5: $\sigma_- = 129,8 \text{ MPa} \Rightarrow \text{OK!}$
 $\tau_- = 11,4 \text{ MPa} \Rightarrow \text{OK!}$

- Section 6: $\sigma_- = 131,1 \text{ MPa} \Rightarrow \text{OK!}$
 $\tau_- = 10,7 \text{ MPa} \Rightarrow \text{OK!}$

In order to get an estimate of the deflection of the beam, the deflection is calculated on basis of moment of inertia at the middle of the angled cut beam. Not in the middle of section 5 as

$$I_y = 4,96\text{E}+07 \text{ mm}^4$$

from the deflection is

$$u_{\max} = 4,86 \text{ mm} \Rightarrow \text{OK!}$$

(*): I have added a factor 1,2 to the deflection formular for straight beams, in order to take into